

VILLAGE OF BREWSTER  
BOARD OF TRUSTEES  
03 OCTOBER 2018  
7:30 P.M.  
REGULAR MEETING  
MINUTES

The Board of Trustees of the Village of Brewster held a regular meeting at 7:30 PM on October 3, 2018 at Village Hall, 50 Main Street, Brewster, NY 10509.

Attendees:

Mayor:

Deputy Mayor and Trustee:

Trustees; Mary Bryde, Tom Boissonnault, George Gaspar

Village Engineer: Todd Atkinson

Village Counsel: Anthony Molé

Clerk & Treasurer: Peter Hansen

Deputy Clerk-Treasurer Michelle Chiudina

Absent:

James Schoenig

Christine Piccini

Pledge to flag.

Notation of Exits

Motions to open the regular meeting, Trustee Bryde 2<sup>nd</sup> all in favor 3 to 0.

**Regular Meeting**

1. DPW Report – DPW Superintendent Domenic Consentino delivered the DPW report. Superintendent Consentino says he will shut the water off at Wells Park and has contacted the plumber to winterize. Mr. Consentino brought two different sections of water pipe that had sprung leaks; one 2" galvanized section from Argonne Road that had been previously patched at one end, and one ¾" copper section from Eastview Avenue. Both have significant deterioration including multiple pin holes. Village Engineer Atkinson says it looks like it could be stray voltage that could be accelerating the degradation of the metal. A brief discussion of addressing the possibilities of the source of stray voltage, such as the use of water lines as grounds by homeowners, ensued and the potential for checking voltage on the next section found leaking, possibly checking voltage at different locations that are accessible, possibly installing isolators between our street mains and the homeowner supply line to prevent transient voltage from affecting our lines. No consensus action was taken. Trustee Bryde asks about the potential candidates for the DPW position. Mr. Consentino says he has four or five to call and possibly interview. Mr. Consentino says hydrant flushing was conducted today on schedule. Trustee Boissonnault motions to accept the DPW report, Trustee Bryde 2<sup>nd</sup> all in favor 3 to 0.
2. Engineer's Report – Village Engineer Todd Atkinson delivered the Engineer's report. Copy attached to these Minutes. Trustee Gaspar motions to accept the July & August Engineer's report, Trustee Boissonnault 2<sup>nd</sup> all in favor 3 to 0.
3. Financial Update - Treasurer Hansen says we are on track with budget and expenditures. We received the New York State Aid to Independent Municipalities (AIM) of \$11,560 but there was no "per capita" check (\$2,300) this year. The Comptroller's website had this to say: "As in several previous years, this year's Executive Budget proposed eliminating Village per Capita Aid (\$1.8 million in SFY 2017-18); unlike previous years, the Enacted Budget accepted the elimination." Treasurer Hansen said with Peaceable Hill water district probably dropping off the system for the 2<sup>nd</sup> half of this fiscal year we could lose approximately \$60,000 in cash income. This makes continued enforcement of on-time payments from the current users' imperative. Treasurer Hansen said he would also recommend we do something about Refuse payment enforcement so those unpaid balances are not left to go to tax relevy. Village Counsel Molé says he will look into Refuse payment enforcement options. Trustee Boissonnault motions to accept the financial report, Trustee Bryde 2<sup>nd</sup> all in favor 3 to 0.
4. Water Repayment Agreements
  - 4.1. Trustee Boissonnault motions to approve the water repayment agreements; account #172 and repayment agreement account #128, Trustee Bryde 2<sup>nd</sup> all in favor 3 to 0.
5. Minutes for approval;
  - 5.1. September 19, 2018 Regular Meeting Minutes – Trustee Bryde motions to approve the September 19, 2018 Minutes, Trustee Gaspar 2<sup>nd</sup> all in favor 3 to 0.

6. Vouchers Payable – Trustee Bryde reviewed the vouchers and found everything in order.

6.1. A -	GENERAL FUND	\$ 13,782.17
6.2. C –	REFUSE & GARBAGE	702.32
6.3. F -	WATER FUND	12,738.14
6.4. G -	SEWER FUND	53,543.31
6.5. T -	TRUST & AGENCY	7,517.62

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Total Vouchers Payable \$88,283.56

Trustee Boissonnault motions to approve vouchers payable as written, Trustee Gaspar 2<sup>nd</sup> all in favor 3 to 0.

7. Other Business

7.1. Trustee Bryde asks Mr. Consentino when the flower pots can be brought down. Mr. Consentino says whenever you want them down. Trustee Bryde said we might look at using the empty baskets for holiday decorations. Trustee Bryde said the Vietnam Memorial Traveling Wall came through the Village around 3:30 pm today headed for Putnam County Veterans' Park and there was a good turnout to watch the procession on Main Street. Trustee Bryde asks about the letter thanking the Town Highway Supervisor for blocking the streets with Town trucks at the Brewster Fall Festival. Clerk Hansen said it had not been addressed as yet but will be produced soon. Trustee Bryde asks about the Village Matters schedule. Clerk Hansen says he received a email from Paloma Domenico and the dates were not conducive to our requirements but he hasn't responded as yet. Trustee Bryde asks how the online billing payments going. Treasurer Hansen and Deputy Clerk-Treasurer Michelle Chiudina say that perhaps a dozen transactions have occurred so far.

7.2. Trustee Boissonnault installed a new lock on the Wells Park kiosk and is leaving the keys with the Village Office. Trustee Boissonnault says we are officially closing Wells Park to the public at dusk on October 8, 2018 because the cold weather is approaching and water pipes could freeze so we have to close the bathrooms and winterize the plumbing.

8. New Business

8.1. Trustee Bryde asks about new sexual harassment policy and training mandated by the State. Clerk Hansen says we will make sure the new policy is adopted as it applies to the Village and says our insurance company will provide training at no charge.

9. Public Comment

9.1. Rick Stockburger thanks the Board for the written nomination in support of the award he received from Brewster Emergency Shelter Partnership for Public Service. Trustee Boissonnault says it was well deserved.

10. Trustee Boissonnault motions to go into executive session to discuss a contract issue with the possibility of action being taken after executive session, Trustee Bryde 2<sup>nd</sup> all in favor 3 to 0.

11. Trustee Boissonnault motions to come out of executive session and resume the regular meeting, Trustee Bryde 2<sup>nd</sup> all in favor 3 to 0.

12. Trustee Gaspar motions to authorize Trustee Boissonnault to sign a letter to the Town of Southeast regarding Peaceable Hill Water District contract termination, Trustee Bryde 2<sup>nd</sup> all in favor 3 to 0.

13. Trustee Boissonnault motions to adjourn, Trustee Gaspar 2<sup>nd</sup> all in favor 3 to 0.





**SKYLANDS TESTING, LLC**

124 Milton Road, Sparta, NJ 07671  
973-729-4824  
973-729-5824 fax

## Transmittal

To: J. Robert Folchetti & Associates, LLC  
31 Sodom Road  
Brewster, NY 10509

Date: August 7, 2018 Project No: 1036  
Project: Tonetta Brook Marvin Ave. Headwall  
Reference: Geotechnical Report  
cc:

Attention: Mr. Todd Atkinson, P.E.

**We Transmit:****the following:****for:**

- |   |   |   |   |
|---|---|---|---|
| <input type="checkbox"/> as per your request  | <input type="checkbox"/> calculations       | <input checked="" type="checkbox"/> your file/use | <input type="checkbox"/> as requested by _____              |
| <input type="checkbox"/> under separate cover | <input checked="" type="checkbox"/> reports | <input checked="" type="checkbox"/> distribution  |   |
| <input checked="" type="checkbox"/> by mail   | <input type="checkbox"/> samples            | <input type="checkbox"/> your approval            | <input type="checkbox"/> as approved by _____               |
| <input type="checkbox"/> by messenger         | <input type="checkbox"/> copies             | <input type="checkbox"/> revision and submission  |   |
| <input type="checkbox"/> by pick up           | <input type="checkbox"/> letters            | <input type="checkbox"/> your review and comment  | <input type="checkbox"/> as submitted for approval by _____ |
| <input type="checkbox"/> by overnight carrier | <input type="checkbox"/> invoice            |   |   |

Copies	Date	Number	Description
3	8-5-2018		Geotechnical Report

Comments:

 RECEIVED ?  
AUG 8 2018  
J. ROBERT FOLCHETTI

Signed: \_\_\_\_\_

Eugene J. Schwarzrock, P.E.

If enclosures are not as noted, please notify us at once



# Geotechnical Report

## **Village of Brewster** Tonetta Brook Marvin Avenue Headwall

Marvin Avenue at Railroad Avenue  
Brewster, New York

August 3, 2018

Prepared for:

**J. Robert Folchetti & Associates**  
31 Sodom Road  
Brewster, NY 10509

Prepared by:

**SKYLANDS ENGINEERING, LLC**  
124 Milton Road  
Sparta, NJ 07871

# Geotechnical Report

## Village of Brewster

### Tonetta Brook Marvin Avenue Headwall

Marvin Avenue at Railroad Avenue  
Brewster, New York


August 3, 2018

Prepared for:

J. Robert Folchetti & Associates  
31 Sodom Road  
Brewster, NY 10509

Prepared by:

SKYLANDS ENGINEERING, LLC  
124 Milton Road  
Sparta, NJ 07871  
Certificate of Authorization No. 0013524

  
Eugene J. Schwabach, Professional Engineer  
New York License No. 077007-1

8-5-18  
Date

Note: It is a violation of NY Education Law Section 7209 for any person to alter any item in this report in any way, unless they are acting under the direction of a Professional Engineer registered in New York. The altering engineer shall affix to this page their seal, the notation "altered by:" followed by their signature and date of alteration, and a specific description of the alteration(s) made.

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### APPENDIX

- Boring Location Plan
- Boring Logs





## INTRODUCTION

This project involves the design and construction of a new headwall and wingwalls in front of the existing headwall and wingwalls at the Marvin Avenue embankment over Tonetta Brook, located just east of Railroad Avenue (Route 53), in the Village of Brewster, Putnam County, New York. Existing headwalls along the north and south side of Marvin Avenue contain this roadway as it passes over the brook. Three (3) triangularly-stacked, 36 in. diameter reinforced concrete pipes at the invert of the brook allow the upstream, open-channel flow to pass through the Marvin Avenue fill and continue into the downstream, open channel section. The upstream (north) headwall is leaking therefore a new headwall is proposed to be constructed a few feet in front of (upstream) the existing headwall, and the interstitial space filled with controlled low-strength material/flowable fill.

The existing headwall runs  $\pm 13$  ft. long along Marvin Avenue, and has two (2) flared wingwalls which are each 8 ft. long. The top of the headwall stops a few inches above the roadway grade and steel guiderail is located immediately behind the headwall.

This report presents the findings of a subsurface investigation prepared specifically for this project, as well as recommendations for design and construction of the proposed improvements.

## GEOLOGY

Based on our review of published geologic data for this area of Westchester County, including the *Surficial Geologic Map of New York - Lower Hudson Sheet*, 1989, by Cadwell, Connally, et. al., this site is expected to be underlain by glacial till consisting of a mixture of grain sizes ranging from clay and silt, to sand, cobbles and boulders. Underlying bedrock is expected to consist of gray hornblende-biotite-quartz-plagioclase layered gneiss with interlayers of marble and possibly granite, based on the *Bedrock Geology of Parts of Putnam and Westchester Counties, New York, and Fairfield County, Connecticut*, 1968, by Prucha, Scotford, and Sneider, and the *(Bedrock) Geologic Map of New York - Lower Hudson Sheet*, 1970, by Rickard, Isachsen, and Fisher.

## SUBSURFACE INVESTIGATION

Soiltesting, Inc. of Oxford, Connecticut performed three (3) borings on July 11, 2018 to identify the subsurface conditions present beneath the proposed headwall. Boring B-1 was drilled  $\pm 20$  ft. NNE of the end of the proposed east wingwall, boring B-3 was drilled  $\pm 10$  ft. east of the end of the proposed east wingwall, and boring B-4 was drilled  $\pm 15$  ft. west of the end of the proposed west wingwall. The borings were sampled at maximum 5 ft. intervals, and boring B-1 was terminated on auger refusal at a depth of 18 ft. Borings B-3 and B-4 were terminated at depths of 18 ft. and 20.5 ft., respectively, after the completion of a 5 ft. rock core in each boring.

All borings were drilled using an ATV-mounted drill rig and a hollow stem auger to advance and maintain the hole. Soil sampling was performed using a 2 in. O.D. split spoon sampler driven by a 140 lb. safety hammer with a 30 in. drop and the number of blows for each 6 in. increment was recorded, in accordance with procedures outlined in ASTM D1586, Standard Test Method for Standard Penetration Test (SPT) and Split-Barrel Sampling of Soils. Bedrock was sampled using a double-tube core barrel in accordance with procedures outlined in ASTM D2113 - Standard Practice for Rock Core Drilling and Sampling.



Soil samples were classified by an experienced geologist in general accordance with D.M. Burmister's "Suggested Test Methods for Identification of Soils" (ASTM, 1958) and bedrock samples were classified according to their rock type, and measured percentage recovery and rock quality designation (RQD).

Groundwater was recorded in the field when it was first encountered in the borings.

A Boring Location Plan and typed boring logs are presented in the Appendix.

## **SUBSURFACE CONDITIONS**

The subsurface conditions encountered beneath this site differ somewhat from the published geologic literature. All three (3) borings encountered brown, dark brown, and black, very loose to loose, granular fill to depths ranging from 8 ft. to possibly 11 ft., with this fill consisting generally of silty fine sand, silty medium to fine sand, cinders and ash, silt, brick fragments, and trace amounts of asphalt in the upper 2 ft. of boring B-4. The cinders and ash were encountered at a depth of 5 ft. in all three (3) borings. Beneath this fill a 3 ft. to 7 ft. thick layer light brown, gray-brown and dark brown, medium dense, silty coarse to fine sand with sporadic gravel was encountered to depths of  $\pm 10$  ft. to 11 ft. Beneath this sand, suspected weathered bedrock and/or boulders were present until the top of bedrock was encountered at depths ranging from  $\pm 15$  ft. to 18 ft. Bedrock appeared to be schist.

Standard penetration test N-values ranged from 2 blows per foot (bpf) to 6 bpf in the granular fill, from estimated 14 bpf to 39 bpf in the virgin sand layer, and  $>70$  bpf in the weathered rock layer. Core samples of bedrock at borings B-3 and B-4 had recoveries of 63% and 95%, and RQDs of 20% and 60%, respectively.

Groundwater was encountered in all three (3) borings from depths ranging from 7.5 ft. to 14 ft. (El.  $\pm 341$  to El.  $\pm 335.5$ ), with the higher elevation being close to the existing water level in the brook.

## **DESIGN RECOMMENDATIONS**

Based on the findings of this subsurface investigation program, it is recommended that the new headwall and wingwalls may be founded on conventional spread footings. The recommended frost depth for this area of New York is 42 in. and normally footings should be founded at or below this depth; however, weathered and/or fractured bedrock is expected above this depth and these materials are generally not frost-susceptible. Therefore footings may be designed using an allowable bearing capacity of 3 tsf on the weathered or fractured bedrock. A coefficient of base sliding of 0.45 is recommended.

Should weathered bedrock not be present at any area beneath the footing at the planned footing elevation, then the footing excavation should either be extended to the top of weathered/fractured bedrock, or 12 in. of clean  $\frac{3}{4}$  in. crushed stone should be placed and compacted below that section of footing to provide uniform bearing.

Post construction settlement is expected to be negligible since vertical loads are minimal and footing design will be governed by sliding. Any settlement will be elastic (instantaneous), with no long term consolidation settlement occurring.



The wall should be designed to ultimately support the loading from the existing wall and Marvin Avenue roadway, since the existing wall will likely continue to deteriorate and need to be supported. The following embankment soil properties are recommended for design:

Moist unit weight of retained soil,	$\gamma_1 = 125$ pcf
Angle of internal friction,	$\phi = 34^\circ$
Lateral earth pressure coefficients:	
Active,	$K_a = 0.28$
Passive,	$K_p = 3.54$
Coeff. of friction (sliding),	$\tan \delta = 0.50$ (CIP concrete on weathered bedrock)

There is no evidence of past slope instability and none is expected under static or seismic loading.

The soils at this site are non-liquefiable based on their suitably high relative density and shallow depth to bedrock.

## CONSTRUCTION RECOMMENDATIONS

Prior to construction all topsoil should be stripped and stockpiled for reuse.

Footings shall not be constructed on frozen or wet subgrade materials. Any frozen or saturated subgrade soil should be removed and/or replaced with clean  $\frac{3}{4}$  in. crushed stone.

The bottom of all footing excavations should be compacted using a minimum of four (4) passes with a Rammax, or similar, trench compactor, as required, and until no further settlement is visible prior to placing structural fill or constructing footings.

Cobbles and boulders are not expected to be encountered during footing excavation; however, any cobbles or boulders encountered during construction should be removed so that no part protrudes into the bottom or sides of foundation excavations.

If organic soils are encountered at the bottom of footing excavations, it should be removed from beneath the footing and replaced with compacted  $\frac{3}{4}$  in. stone. Organic soils should not be used as structural backfill, but should be removed offsite.

If NYSDOT Standard Specifications are not utilized, structural fill below normal water elevation should consist of clean,  $\frac{3}{4}$  in. crushed stone placed and compacted in the dry. Structural fill above water may consist of stone or well-graded, coarse to fine sand and/or gravel with a maximum 10% non-plastic fines (material passing a No. 200 sieve) and free of organics and other deleterious materials. Aggregate size should be limited to no bigger than 2 in. in the largest dimension. It is estimated that none of the in situ materials will be suitable for reuse as structural fill due to the silt content and presence of deleterious material. Representative samples of any proposed fill material should be tested for gradation and moisture-density relationship prior to use to confirm its suitability.

Structural fill material should be placed in maximum 10 in. loose lifts beneath the wall footings, and 12 in. elsewhere, and it should be compacted to 95% of its maximum dry density at optimum moisture content as determined by the Modified Proctor Density Test (ASTM D1557). These operations should be performed under full-time geotechnical inspection and testing by either the Sand Cone Method (ASTM



D1556), Nuclear Density Gauge (ASTM D2922 and D3012), or other moisture/density test methods. These density tests should be performed by an experienced geotechnical inspector at sufficient frequency and spacing to ensure proper compaction, with the following criteria suggested as guidelines:

Location	Frequency of Testing
Structural fill beneath and adjacent to structures	min. 2 tests per lift min. 3 tests per day
Utility trenches	1 test every 50-100 LF per lift min. 3 tests per day

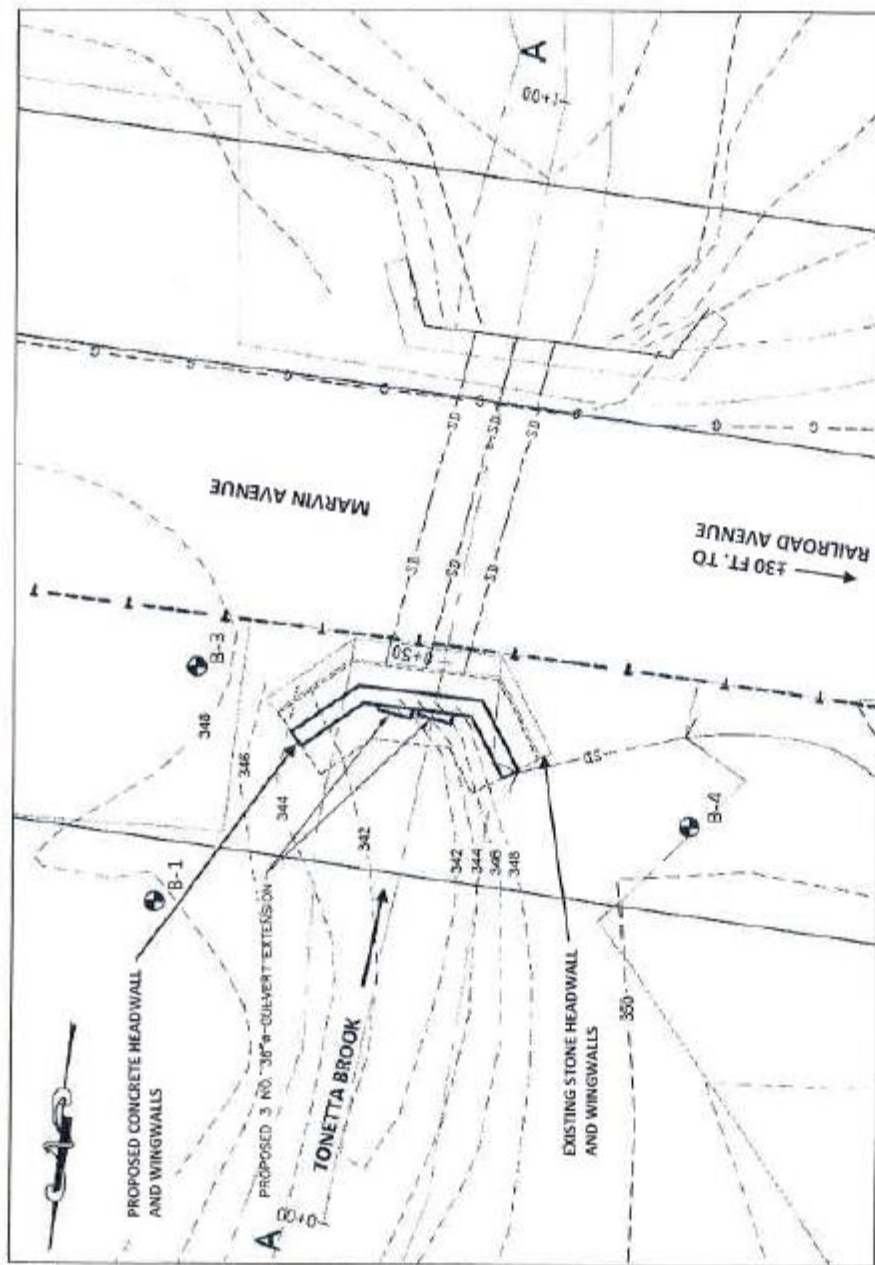
For excavations that extend deeper than 5 ft., sheeting, shoring, sloping, or benching of the excavation sidewalls is required per OSHA standards. Depending on location, all options may be suitable for this project, however cantilevered sheeting will likely not be suitable due to the presence of shallow bedrock. Based upon the material characteristics and estimated strength of the soils encountered during the subsurface exploration, the soil present on site may be assumed to be Type C and should be sloped at a 1.5H:1V (34°) per OSHA requirements. For the design of temporary sheeting or shoring, the following additional soil properties are recommended for the in situ soils not beneath Marvin Ave.

Moist unit weight of retained soil,	$\gamma_t = 115$ pcf
Angle of internal friction,	$\phi = 27^\circ$
Lateral earth pressure coefficients:	
Active,	$K_a = 0.38$
Passive,	$K_p = 2.66$

All sheeting, shoring and bracing shall be designed by a professional engineer registered in the State of New York. Note that shorter, unbraced excavations will experience localized instability (i.e., sloughing) if left open for more than a day due to the open gradation of the material and expected rapid loss of moisture. To reduce the severity of this sloughing, such excavations should be covered with plastic sheeting for protection from rainfall and moisture changes. The layer of cinders and ash are not expected to have any stand-up time and will likely slough immediately.

It is recommended that all foundation and subgrade preparation procedures be inspected by a qualified geotechnical engineer experienced with this type of construction.

## APPENDIX



# **LEGEND**

-  BORING
-  B-1

**SCALE**  
N.T.S.

## **BORING LOCATION PLAN**

**TONETTA BROOK MARVIN AVENUE HEADWALL**  
MARVIN AVENUE AT RAILROAD AVENUE  
BREWSTER, NEW YORK

**SKYLANDS ENGINEERING, LLC**

124 MILTON ROAD  
SPARTA, NJ 07871

CERTIFICATE OF AUTHORIZATION NO. 0013524

DATE: 8-3-2018

### **NOTE:**

1. BASE PLAN TAKEN FROM "PLANS AND PROFILE AND ELEVATIONS", DEC 2017, SUPPLIED BY J. ROBERT FOLCHETTI & ASSOCIATES, LLC
2. BORING B-2 NOT DRILLED



## Boring Logs

<b>SOILTESTING, INC.</b> 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850		CLIENT: <b>J. Robert Folchetti &amp; Associates</b> PROJECT NO. <b>G127-1049-18</b> PROJECT NAME <b>Marvin Avenue over Tonetta Brook at Railroad Avenue</b>		SHEET <u>1</u> OF <u>1</u> HOLE NO. <b>B-1</b>	
FOREMAN - DRILLER <b>PD/jk</b> INSPECTOR		LOCATION <b>Brewster, NY</b>		BORING LOCATIONS per Plan	
GROUND WATER OBSERVATIONS AT <u>6</u> FT AFTER <u>0</u> HOURS AT <u>  </u> FT AFTER <u>  </u> HOURS		TYPE SIZE I.D. HAMMER WT. HAMMER FALL		CASING HSA 3 3/4" 140# 30" dia SAMPLER SS 1 3/8" BIT dia CORE BAR NQ2 2" OFFSET DATE START 7/11/18 DATE FINISH 7/11/18 SURFACE ELEV. ±348 GROUND WATER ELEV. ±340	

DEPTH	CASING BLOWS PER FOOT	SAMPLE				BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6 - 12 - 18	CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC					
		1	ss	24"	10"	20"	1 2			
							3 5			
5										
		2	ss	24"	7"	70"	2 3			
							3 5			
10										
		3	ss	10"	10"	117"	6 7		11'0"	Drk Brn VF-FMC SAND & FC GRAVEL, sm silt
							71 100/1"			partially decomposed / fractured BEDROCK or BOULDERS, sm VF-F sand, silt
15										
		4	ss	24"	12"	170"	21 20			partially decomposed BEDROCK / ROCK frags
							54 72			
20									18'0"	AUGER REFUSAL
										E.O.B. 18'0"
25										
30										
35										
40										

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO <u>  </u> FT. USED <u>  </u> CASING THEN <u>  </u> CASING TO <u>  </u> FT	HOLE NO. <b>B-1</b>
A = AUGER UP = UNDISTURBED PISTON    T = THINWALL    V = VANE TEST WOR = WEIGHT OF RODS    WOH = WEIGHT OF HAMMER & RODS SS = SPLIT TUBE SAMPLER    H.S.A. = HOLLOW STEM AUGER PROPORTIONS USED: TRACE = 0 - 10%    LITTLE = 10 - 20%    SOME = 20 - 35%    AND = 35 - 50%	
C = COARSE M = MEDIUM F = FINE	

<b>SOILTESTING, INC.</b> 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850				CLIENT: <b>J. Robert Folchetti &amp; Associates</b>				SHEET <u>1</u> OF <u>1</u> HOLE NO. <b>B-3</b>	
				PROJECT NO. <b>G127-1049-18</b>					
				PROJECT NAME <b>Marvin Avenue over Tonetta Brook at Railroad Avenue</b>				BORING LOCATIONS per Plan	
FOREMAN - DRILLER <b>PD/jk</b>				LOCATION <b>Brewster, NY</b>					
INSPECTOR								OFFSET	
GROUND WATER OBSERVATIONS AT <u>7.6</u> FT AFTER <u>0</u> HOURS AT <u>  </u> FT AFTER <u>  </u> HOURS				TYPE SIZE I.D. HAMMER WT. HAMMER FALL		CASING <b>HSA</b> <b>3 3/4"</b> <b>140#</b> <b>30"</b>		SAMPLER <b>SS</b> <b>1 3/8"</b> <b>BIT</b> <b>dia</b>	
				CORE BAR <b>NQ2</b>				DATE START <b>7/11/18</b> DATE FINISH <b>7/11/18</b> SURFACE ELEV. <b>±348.5</b> GROUND WATER ELEV. <b>±341</b>	

DEPTH	CASING BLOWS PER FOOT	SAMPLE				DEPTH @ BOT	BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE)		CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC		0 - 6	6 - 12				
5		1	ss	24"	10"	26"	2	2		moist/moist v loose		2" ASPHALT Dk Bm Blk VF-F SAND, sm silt, lil F gravel
						2	2					
		2	ss	24"	12"	70"	1	1		moist/moist v loose		CINDERS / ASH & Dk Bm Bm SILT & VF SAND, sm Bm Li Gry FMC sand ( fill )
							1	2				
10											8'10"	
		3	ss	24"	14"	120"	3	3		wet compact	10'8"	Li Bm Li Gry SILT & VF-F SAND
						14	13		11'0"		Dk Bm VF-FM SAND, sm silt	
									11'8"		Bm Li Bm Gry VF-FMC SAND, lil F gravel	
15		1	d	60"	38"	18'0"	RQD = 20% Rec = 63%		2.5		13'0"	fractured ROCK or BOULDERS BEDROCK ( Schist )
									2			
									2.25			
									2.5			
20											18'0"	E.O.B. 18'0"
25												
30												
35												
40												

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO <u>  </u> FT. USED <u>  </u> CASING THEN <u>  </u> CASING TO <u>  </u> FT.	HOLE NO. <b>B-3</b>
A = AUGER UP = UNDISTURBED PISTON      T = THINWALL      V = VANE TEST WOR = WEIGHT OF RODS      WOH = WEIGHT OF HAMMER & RODS SS = SPLIT TUBE SAMPLER      H.S.A. = HOLLOW STEM AUGER PROPORTIONS USED: TRACE = 0 - 10%    LITTLE = 10 - 20%    SOME = 20 - 35%    AND = 35 - 50%	
C = COARSE M = MEDIUM F = FINE	



<b>SOILTESTING, INC.</b> 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850		CLIENT: <b>J. Robert Folchetti &amp; Associates</b> <hr/> PROJECT NO. <b>G127-1049-18</b> PROJECT NAME <b>Marvin Avenue over Tonetta Brook at Railroad Avenue</b>		SHEET <b>1</b> OF <b>1</b> HOLE NO. <b>B-4</b>	
FOREMAN - DRILLER <b>PD/jk</b> <hr/> INSPECTOR		LOCATION <b>Brewster, NY</b>		BORING LOCATIONS per Plan	
GROUND WATER OBSERVATIONS AT <u>14</u> FT AFTER <u>0</u> HOURS AT <u>  </u> FT AFTER <u>  </u> HOURS		TYPE <b>HSA</b> SIZE I.D. <b>3 3/4"</b> HAMMER WT. <b>140#</b> HAMMER FALL <b>30"</b>		SAMPLER <b>SS</b> CORE BAR <b>NQ2</b> DATE START <b>7/11/18</b> DATE FINISH <b>7/11/18</b> SURFACE ELEV. <b>±349.5</b> GROUND WATER ELEV. <b>±335.5</b>	

DEPTH FEET	CASING BLOWS PER FOOT	SAMPLE				BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6 - 12 - 18				CORE TIME PER FT (MIN)	DENSITY OR CONSIST	STRATA CHANGE DEPTH	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		NO	Type	PEN	REC	DEPTH @ BOT	0 - 6	6 - 12	12 - 18				
5		1	ss	24"	8"	20"	2	3			moist loose		4" ASPHALT Bm Drk Bm Blk VF-FM SAND, sm silt, FC gravel, asphalt frags
						2	2						
10		2	ss	24"	9"	70"	2	1			moist v loose		Blk CINDERS & ASH, sm Bm Drk Bm VF-FM sand, lit FC gravel, silt, cobbles (ll)
						1	2						
15		3	ss	24"	14"	120"	10	17			moist dense		90" Gry Bm VF-F SAND, lit silt ( possible highly decomposed ROCK )
							22	34					
20		4	ss	3"	2"	153"	100/3"				wdt v dense		140" partially decomposed / fractured BEDROCK
		1	c	62"	57"	206"	RQD = 60%		2.5				156" AUGER REFUSAL
							Rec = 95%		2.5				206" BEDROCK ( Schist )
25													
30													
35													
40													

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO	FT.	USED	CASING	THEN	CASING TO	FT.	HOLE NO.	B-4
A = AUGER    UP = UNDISTURBED PISTON    T = THINWALL    V = VANE TEST WOR = WEIGHT OF RODS    WOH = WEIGHT OF HAMMER & RODS SS = SPLIT TUBE SAMPLER    H.S.A. = HOLLOW STEM AUGER PROPORTIONS USED: TRACE = 0 - 10%    LITTLE = 10 - 20%    SOME = 20 - 35%    AND = 35 - 50%								
C = COARSE M = MEDIUM F = FINE								

1. <u>GENERAL INFORMATION</u>		
Report No: 8 of 2018	Date: 8/15/2018	Contract No:
Facility Name: <b>VOB / PLANNING BOARD REVIEW AND CONSULTATION</b>		

Page 1 of 1

**VILLAGE OF BREWSTER  
MONTHLY PROGRESS REPORT**

<b>1. GENERAL INFORMATION</b>		
Report No: 8 of 2018	Date: 8/15/2018	Contract No:
Facility Name: <b>VOB / EPA Stormwater Phase II Regulations</b>		

<b>2. ENGINEERS STATUS OF PROJECT</b> (Narrative description of the Upgrade Project tasks (and subtasks) undertaken by the Engineer as set forth in the Engineer's Contract and Scope of Work including :	
<b>a.</b>	<b>Activities completed this month (attach additional pages as needed):</b>
	<ul style="list-style-type: none"> <li>▪ Maintained monthly stormwater maintenance/good housekeeping reports quantifying the number of pounds of litter and the amount of sand cleaned up throughout the Village.</li> </ul>
<b>b.</b>	<b>Status of activities in progress this month (attach additional pages as needed):</b>
	<ul style="list-style-type: none"> <li>▪ Continue to assist Village with operation and maintenance issues discussed last month.</li> <li>▪ Awaiting comments, if any, from the NYSDEC on the MS4 Annual Report.</li> </ul>
<b>c.</b>	<b>Activities scheduled (attach additional pages as needed):</b>







VILLAGE OF BREWSTER  
MONTHLY PROGRESS REPORT

1. <u>GENERAL INFORMATION</u>		
Report No: 9 of 2018	Date: 9/19/2018	Contract No:
Facility Name: <b>VOB / PLANNING BOARD REVIEW AND CONSULTATION</b>		

2.	<u>ENGINEERS STATUS OF PROJECT</u> (Narrative description of the Upgrade Project tasks (and subtasks) undertaken by the Engineer as set forth in the Engineer's Contract and Scope of Work including:
a.	Projects reviewed this month (attach additional pages as needed)
	<ul style="list-style-type: none"> <li>530 North Main LLC – 3.75 hours</li> <li>162 Main Street - 3.75 hours</li> </ul>
b.	Status of Planning Board projects (attach additional pages as needed):
	<ul style="list-style-type: none"> <li>Route 22 Brewster LLC (Old Getty Station) – NYCDEP Stormwater Issue</li> <li>538 North Main Street – Construction Ongoing/Amendment being proposed</li> </ul>
c.	Activities scheduled (attach additional pages as needed):
	<ul style="list-style-type: none"> <li>Continue plan review and meetings for B.O.S. Land Development</li> <li>Perform site inspections on Route 22 Brewster LLC (Old Getty Station) Site, when needed.</li> <li>Perform site inspection at 538 North Main Street, when needed.</li> <li>530 North Main Street Planning Review, Ongoing</li> <li>162 Main Street Planning Review, Ongoing</li> </ul>

**VILLAGE OF BREWSTER  
MONTHLY PROGRESS REPORT**

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